



**Jake Hiller, Ph.D.**

Assistant Professor

**Michigan Technological University**

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**MichiganTech**

**CHARACTERIZATION OF RECYCLED CONCRETE  
AGGREGATES FOR REUSE IN RIGID PAVEMENTS**

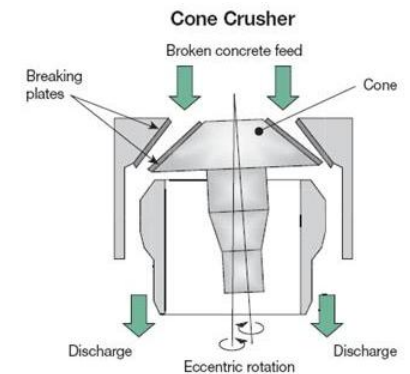
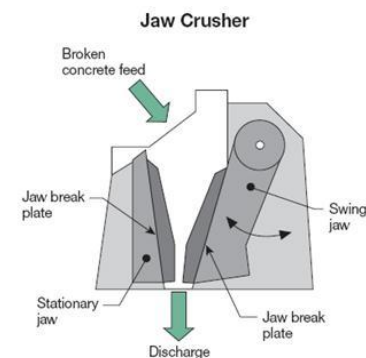
# ACKNOWLEDGEMENTS

- ◎ Michigan Department of Transportation (MDOT)
- ◎ Minnesota Department of Transportation (Mn/DOT)
- ◎ MTU Tomasini Fund
- ◎ Students
  - ◎ Rita Lederle
  - ◎ Morgan Hansen
  - ◎ Cory Shorkey



# RECYCLED CONCRETE AGGREGATES (RCA)

- ◎ RCA is old concrete that has been removed, crushed, and sized for reuse
- ◎ Old aggregates as well as old mortar and some unhydrated cement
- ◎ RCA can contain contaminants
- ◎ RCA differs depending on crushing process



# IMPETUS FOR STUDY

- ◎ RCA use in concrete goes back to post WWII Europe
- ◎ MDOT was pioneer of RCA use in rigid pavements in the USA in 1980's
  - ◎ 1050 lane-km constructed
  - ◎ Moratorium for use in rigid pavements in 1991
- ◎ Costs from landfill, transportation, and quality aggregates has brought issue back into play
- ◎ Sustainability



# I-94 WEST OF KALAMAZOO, MICHIGAN, USA

- ◎ JPCP after 10 years
- ◎ Shrinkage cracking
- ◎ Wide joints and cracks
- ◎ Low load transfer



# SOME RCA ISSUES FOR REUSE IN CONCRETE SURFACE LAYER

- ⊙ Varied crushing processes and high fines
- ⊙ D-cracking and Material-related distress potential
  - ⊙ High alkalinity of water runoff
- ⊙ Leachates (calcium) and high pH for base material
- ⊙ High absorption (mortar / unhydrated cement)
- ⊙ Less volumetric stability
  - ⊙ Shrinkage, creep, and carbonation

# LABORATORY STUDY

- ◎ Aggregate Characterization
    - ◎ Absorption capacity
    - ◎ Specific gravity
- Using three methods**
- ◎ Address RCA in comparison with virgin aggregate concrete
    - ◎ Hardened air content
    - ◎ Shrinkage (drying, autogenous, and restrained)

# COARSE AGGREGATE SOURCES

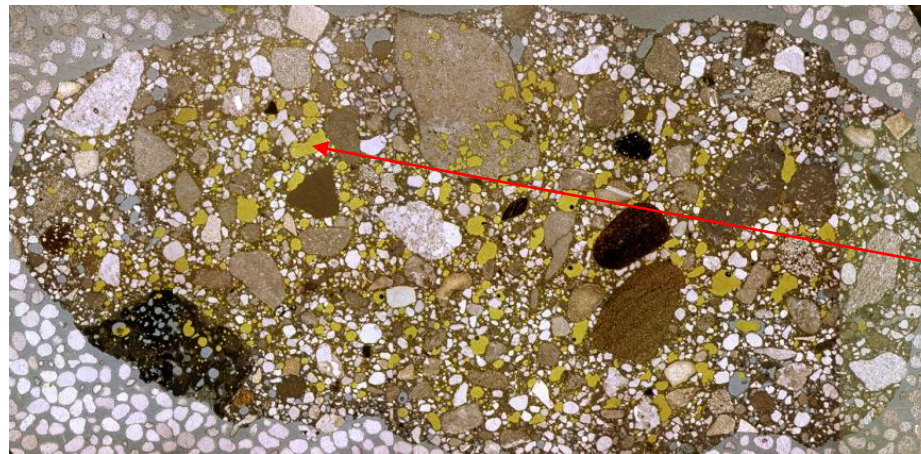
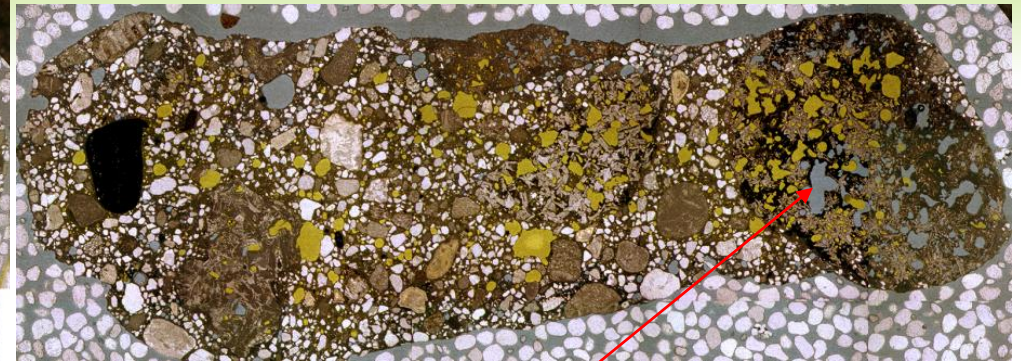
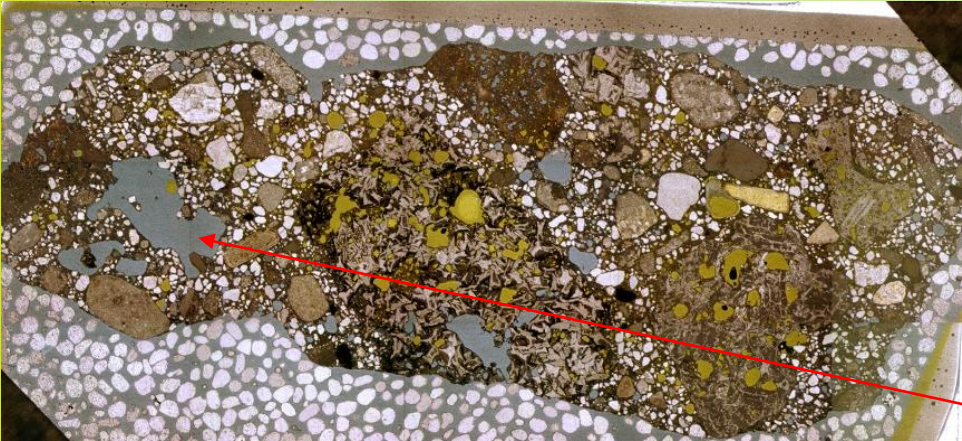
- ◎ Natural aggregates
  - ◎ Crushed gravel
- ◎ RCA (with original aggregate type)
  - ◎ RCA limestone
  - ◎ RCA blast furnace slag
  - ◎ RCA crushed gravel
  - ◎ Recycled RCA crushed gravel (3G/Twice recycled)
- ◎ Fine aggregate → natural sands



# DIFFERENCES IN ABSORPTION CAPACITY AND POROSITY

- ◎ Standard ASTM C127
  - ◎ 24-hour absorption
  - ◎ Visual assessment of SSD → High variability
- ◎ Helium Pycnometer and envelope density analyzer (EDA)
  - ◎ Automated
  - ◎ Assess water absorption?
- ◎ Image Analysis

# IMAGE ANALYSIS - AGGREGATE THIN SECTIONS



Surface  
porosity

Internal  
porosity

**Using  
Impregnated  
resins with  
specified  
viscosities**

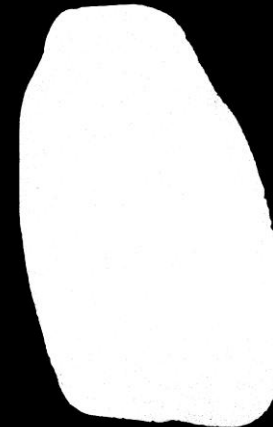
# IMAGE ANALYSIS

- ◎ Imaging software to assess
  - ◎ Pore sizes
  - ◎ Locations
  - ◎ Distribution
- ◎ Leads into future research in moisture diffusivity and poromechanics

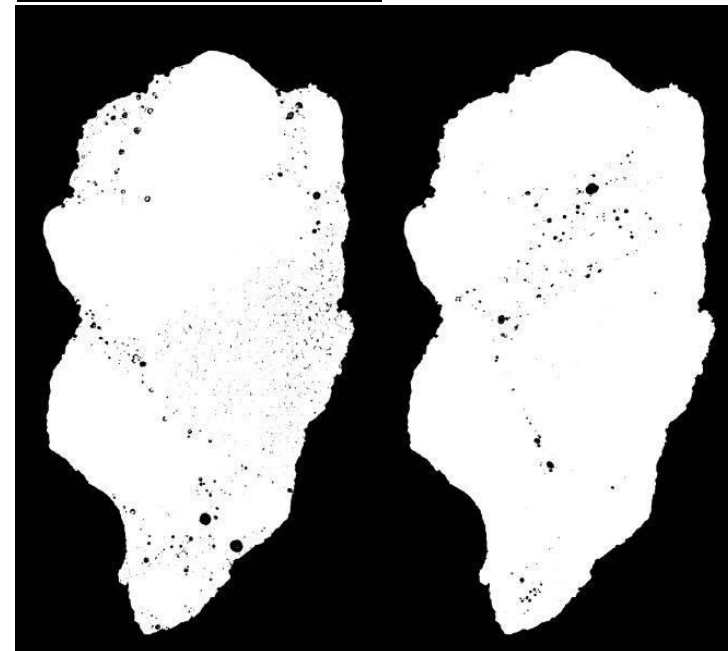
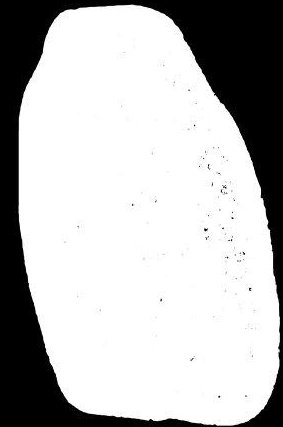
Original Aggregate



Surface Porosity



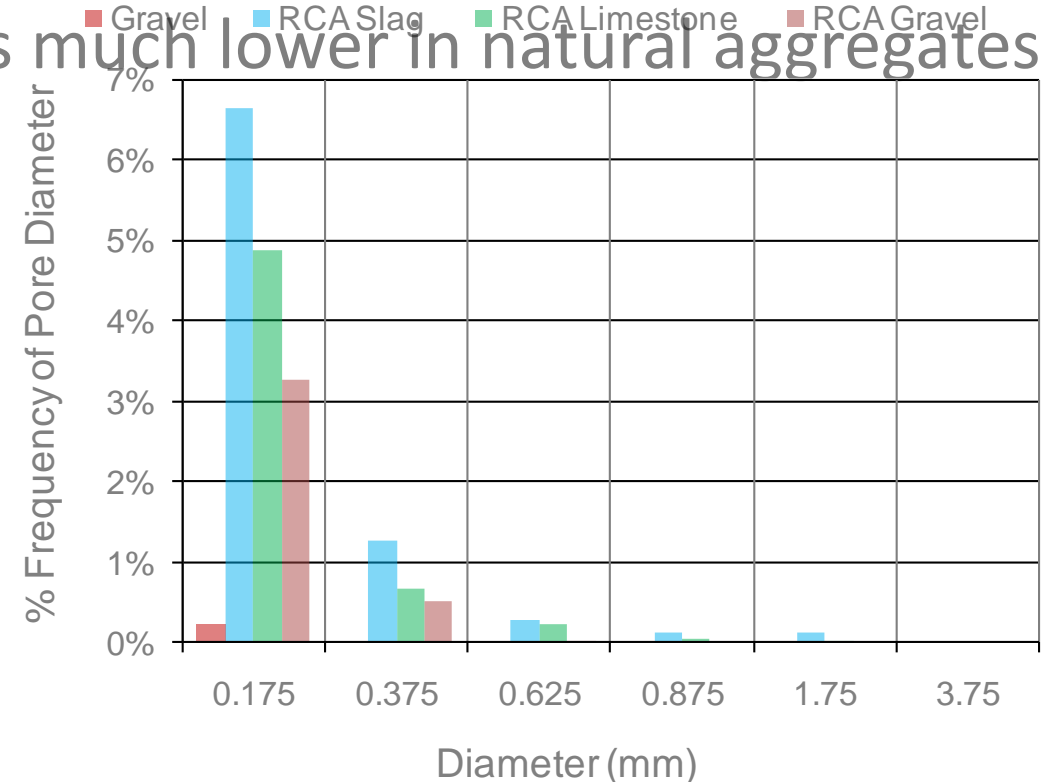
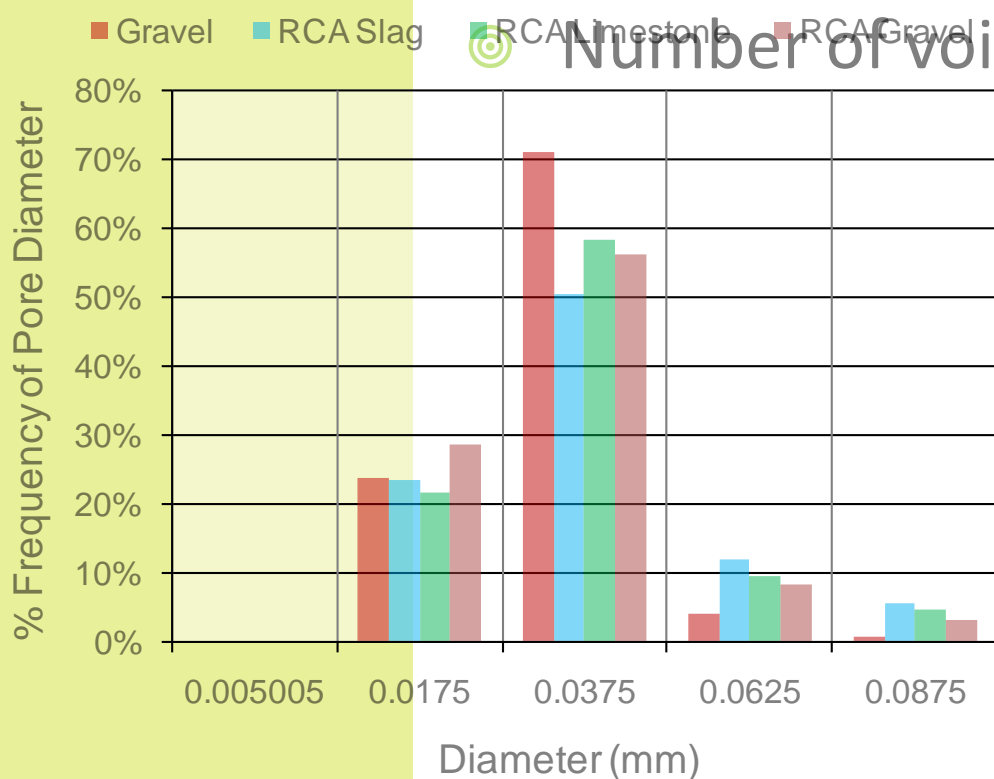
Internal Porosity



# PORE SIZE DISTRIBUTION OF AGGREGATES

🎯 Natural aggregates void of larger Feret's diameter

🎯 Number of voids much lower in natural aggregates

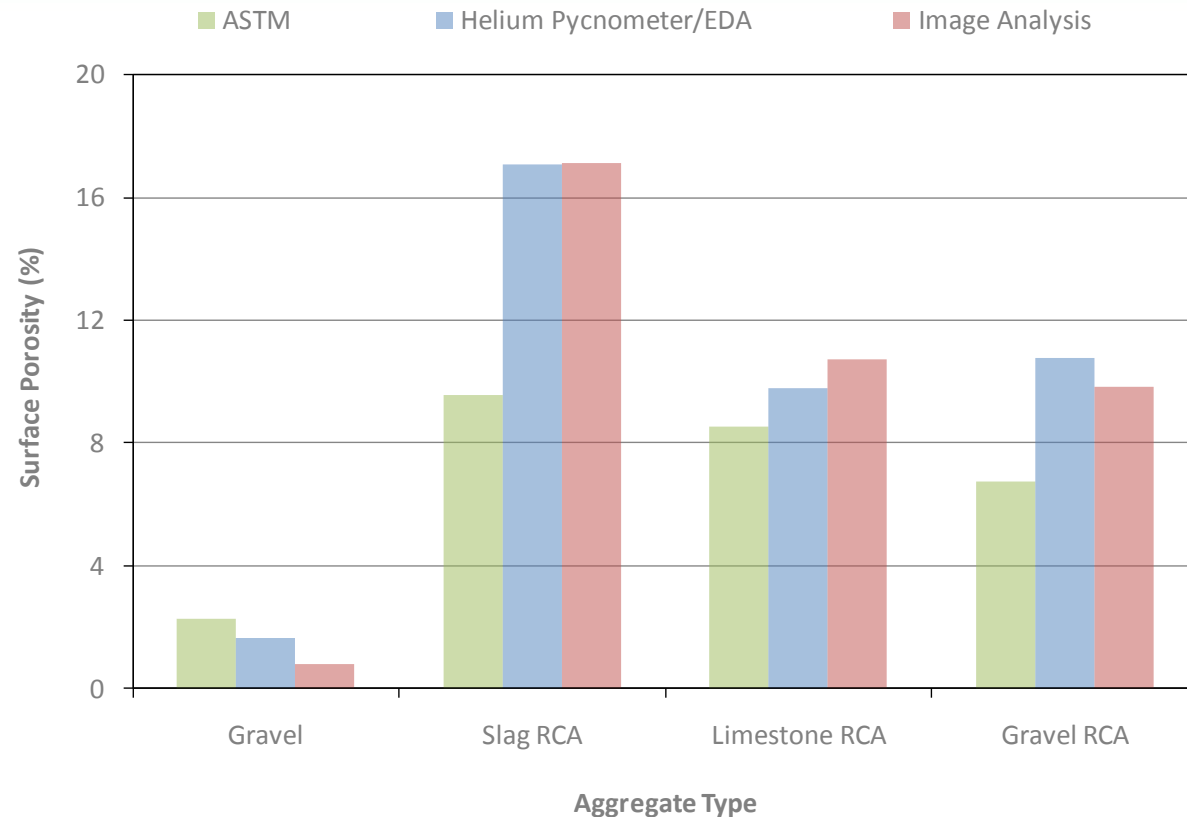




# SURFACE POROSITY BY MULTIPLE METHODS

$$A = \frac{n}{G_B}$$

- Image Analysis matches He Pyc /EDA
- Does ASTM C127 capture porosity/AC for highly porous aggs?



# RCA CONCRETE MIX PROPORTIONING

- ◎ 0.42 water-to-cement ratio
  - ◎ No SCMs
- ◎ 362 kg cement per m<sup>3</sup> of concrete
- ◎ 72% bulk volume of aggregate to vol. of concrete
  - ◎ Paving mixes high in coarse aggregate content
- ◎ Air entrained (target= 6.5%, range of 5.5 – 8.5%)
- ◎ Target slump 50mm
  - ◎ Monitor fresh properties over time

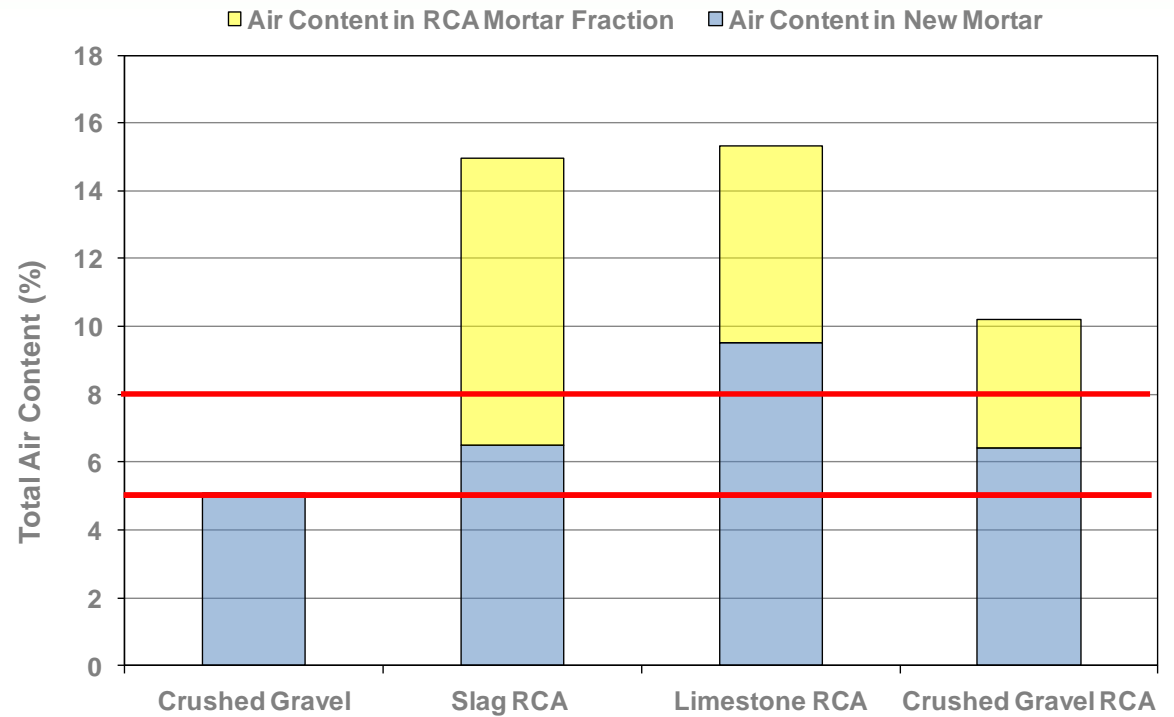


# FRESH PROPERTIES

- ⊙ Measured slumps consistently higher than expected for RCA concretes
  - ⊙ Met specs for natural aggregate concretes
- ⊙ Slump loss was more dramatic in RCA concrete
  - ⊙ Shorter window of workability
  - ⊙ Harsh mixes with poorer consolidation
  - ⊙ Superplasticizers helped to some degree
- ⊙ Fresh air content specification difficult to meet

# HARDENED AIR CONTENT

- ◎ RCA occupies same bulk volume
- ◎ Contains more void space
- ◎ Moisture / chloride movement



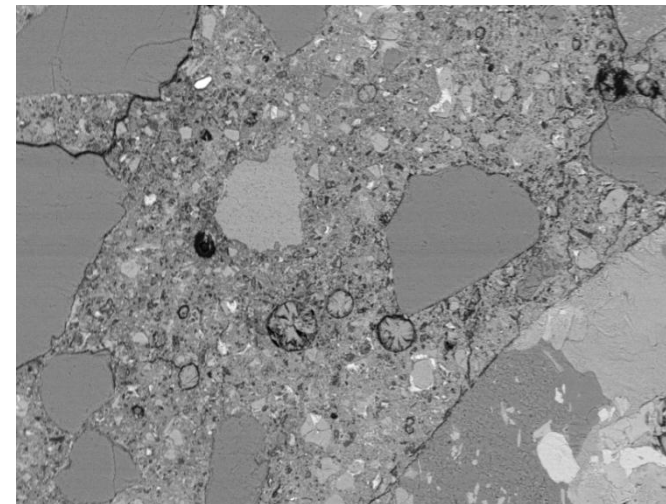
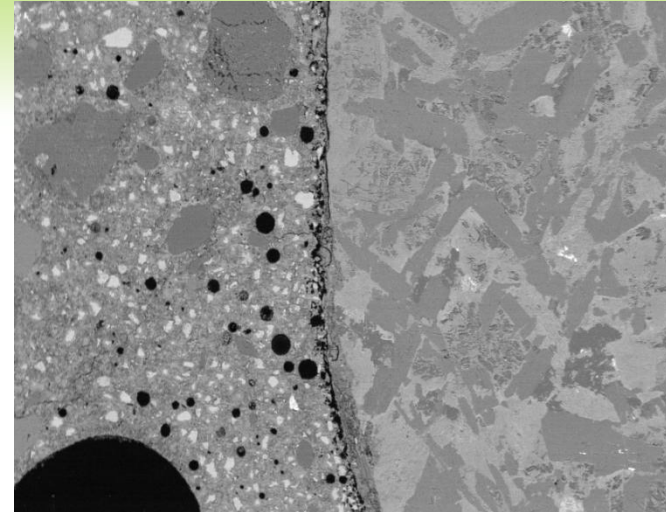
# AIR VOID SYSTEM QUALITY

- ◎ RCA concretes showed better air void system connectivity in general
- ◎ Dependent on old concrete's quality
- ◎ Counterintuitive

Aggregate	Void Frequency	Within Criteria	Specific Surface Area (mm <sup>-1</sup> )	Within Criteria	Spacing Factor (mm)	Within Criteria
Crushed Gravel	0.187	N	16.3	N	0.303	N
Slag RCA	0.339	Y	18.4	N	0.237	N
Limestone RCA	0.522	Y	23.8	N	0.140	Y
Crushed Gravel RCA	0.509	Y	29.8	Y	0.153	Y

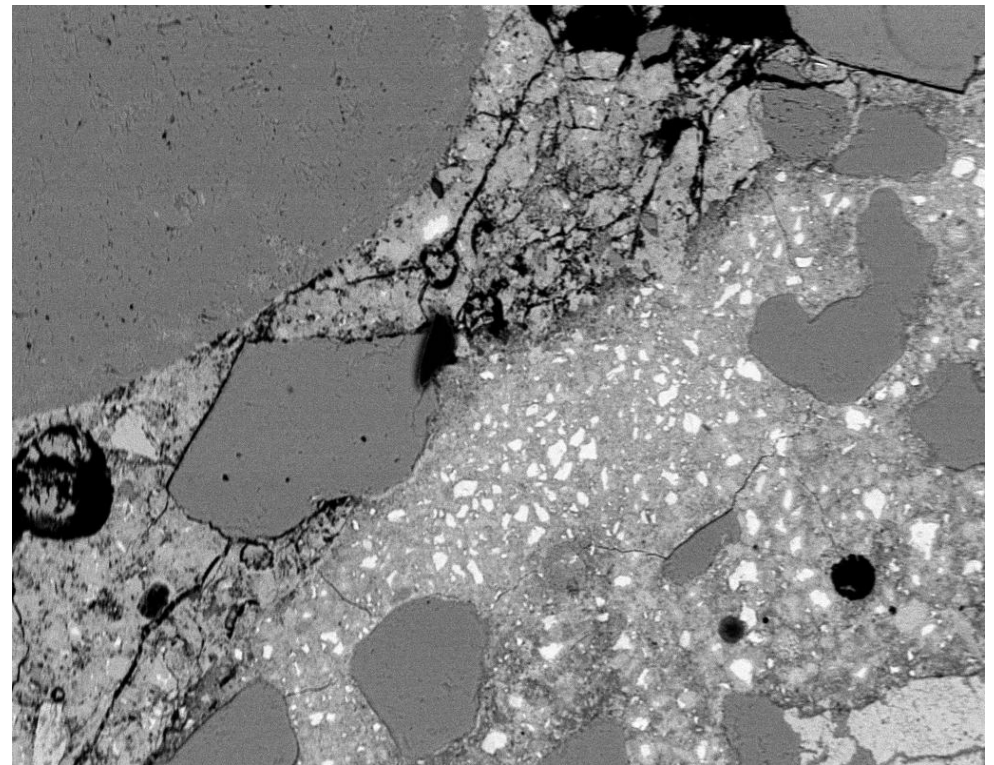
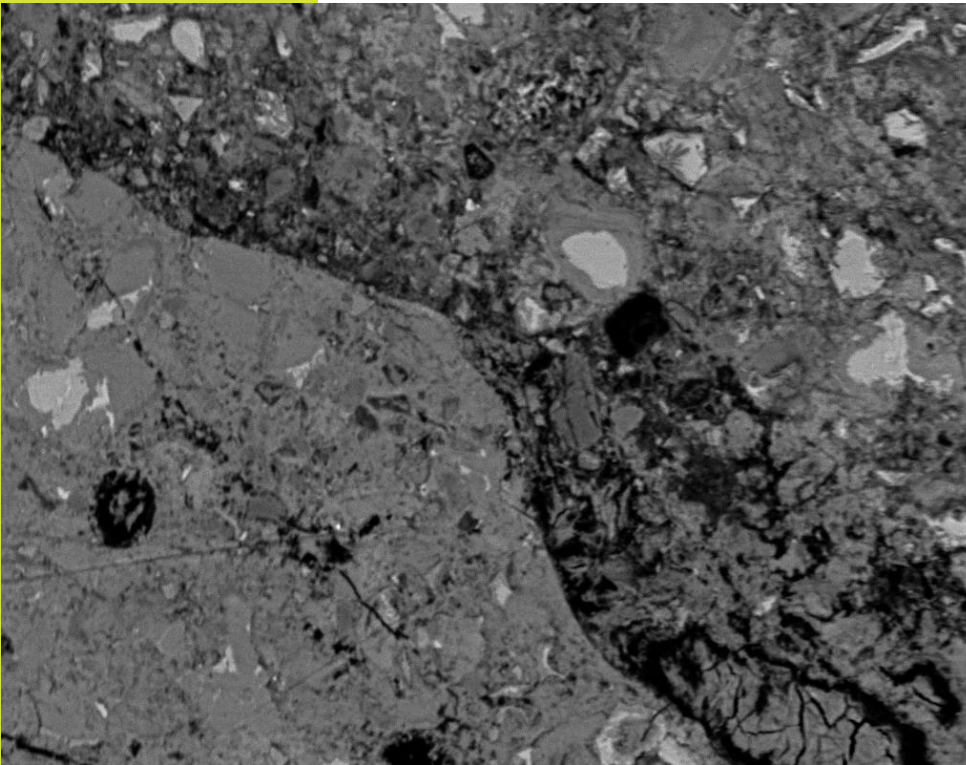
# OPTICAL AND SCANNING ELECTRON MICROSCOPY

- ◎ Porous interfacial transition zones (ITZ)
- ◎ Ettringite filled pores of 3G concrete samples
  - ◎ Good performance of 3G concrete
- ◎ Microcracking in old mortar from crushing process
  - ◎ Reduced fracture resistance



# DISTINCT MORTAR PHASES

- ◎ Some evidence of multiple ITZs





# SHRINKAGE SPECIMENS



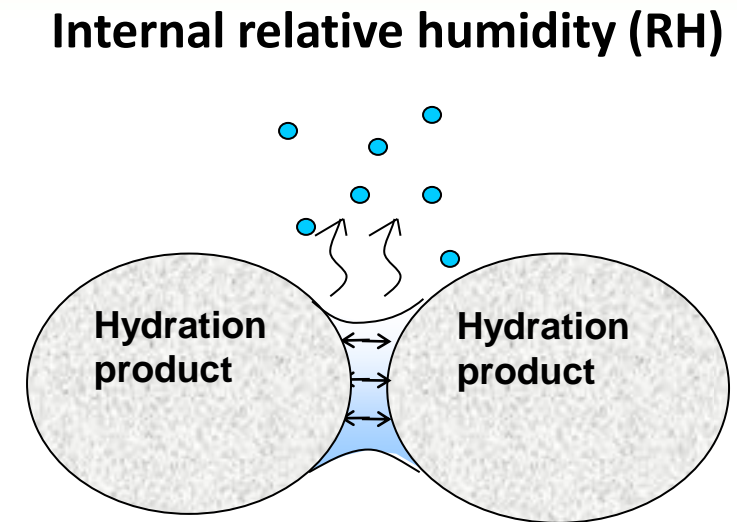
- ◎ 1-D shrinkage
  - ◎ ASTM C 157
  - ◎ Sealed (autogenous)
  - ◎ Unsealed
  - ◎ Difference is drying shrinkage
- ◎ Stored at constant relative humidity, then switched after 1 year, then every 30-45 days





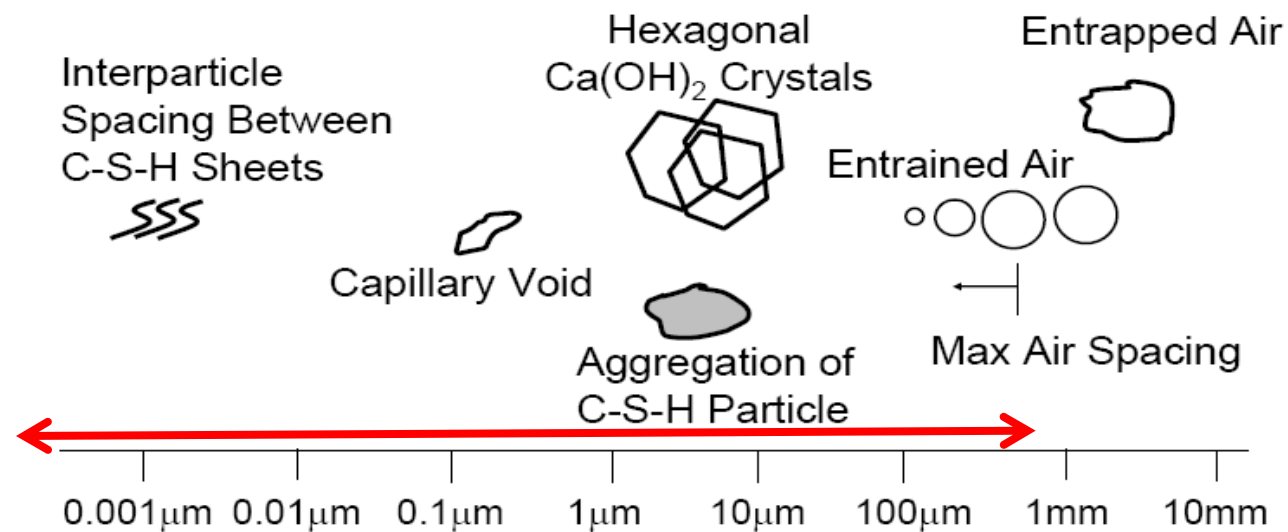
# MECHANISM OF SHRINKAGE

- ③ Shrinkage in concrete is dominated by capillary surface tension mechanisms
- ③ As water leaves pore system, curved menisci develop, creating reduction in RH and “vacuum” within the pore fluid



# VOIDS IN CONCRETE

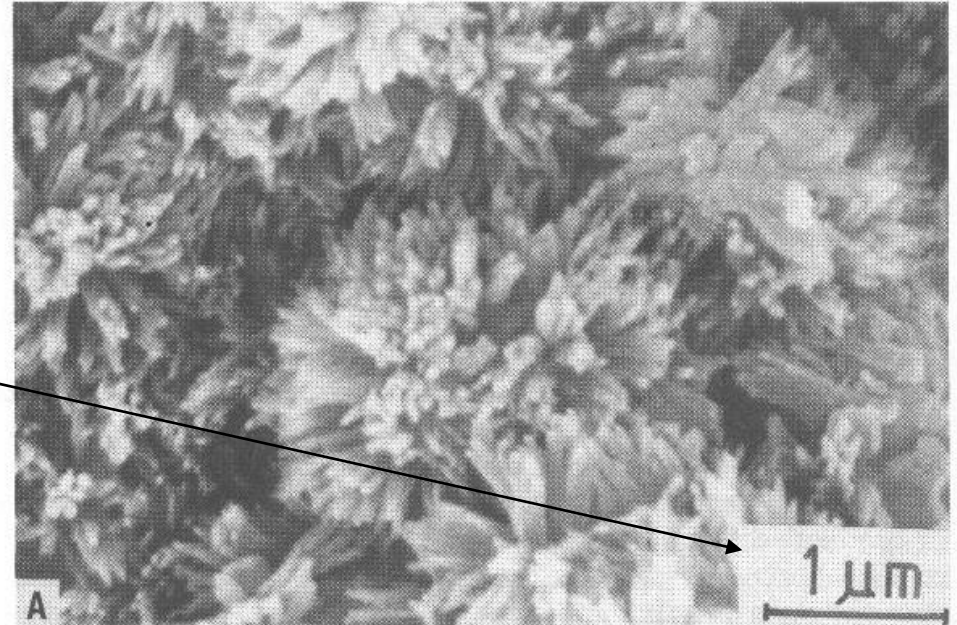
- ◎ RCA concrete contains higher percentage of entrained air through gel pores
- ◎ Affects concrete durability and structural properties



# VISUALIZING THE SCALE OF SHRINKAGE MECHANISMS

- ◎ Capillary stresses present in pores with radii between 2-50 nm

Note the dimensions



- ◎ C-S-H makes up ~70% of hydration product
- ◎ Majority of capillary stresses present in C-S-H network

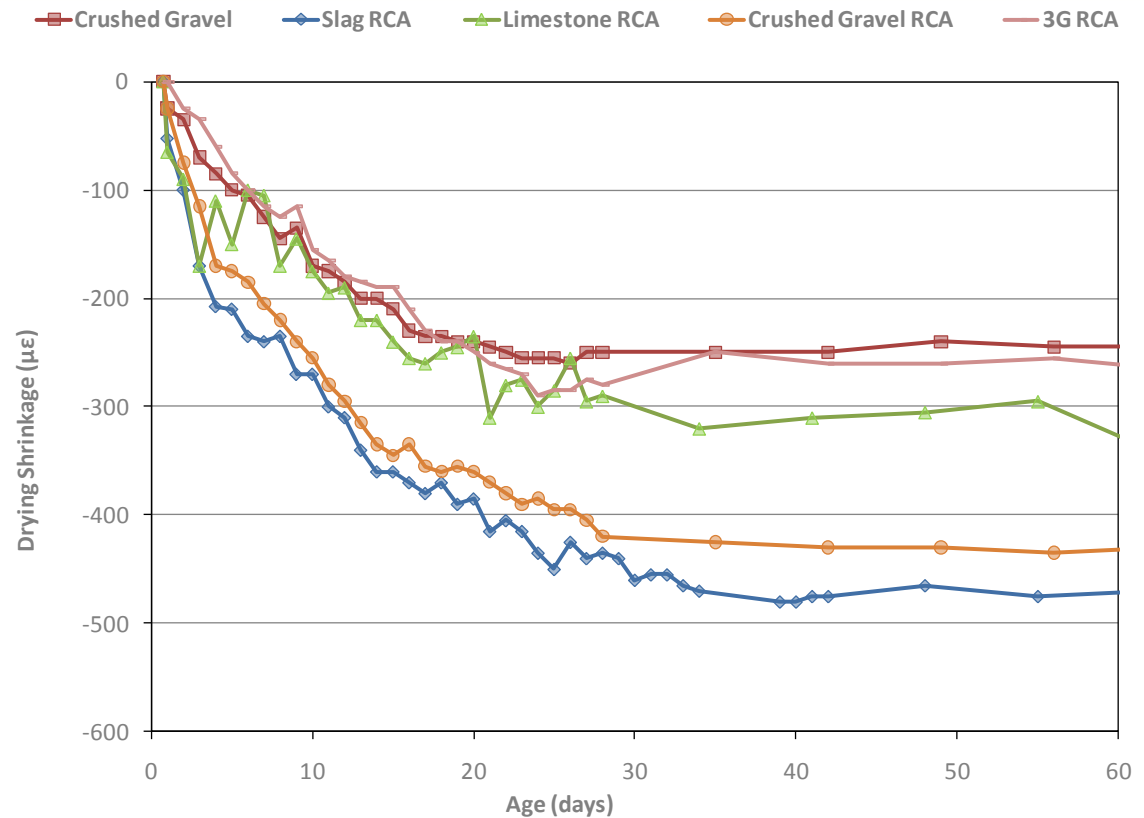
Scanning electron micrograph from Taylor "Cement Chemistry" (originally taken by S. Diamond 1976)

# DRYING SHRINKAGE IN RCA CONCRETE

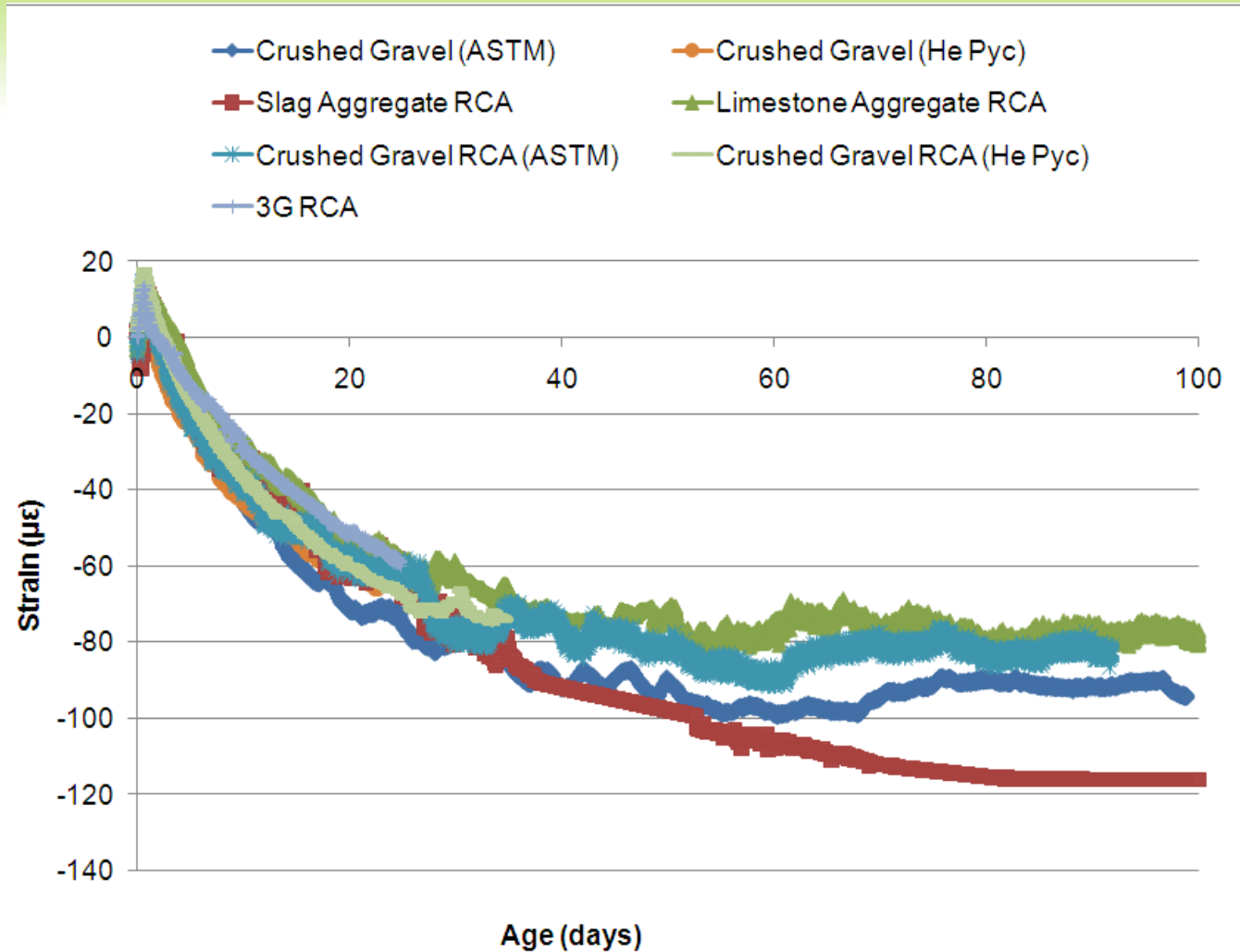
- ◎ Thought to be a driving force in deterioration of JPCPs in many cases
  - ◎ Higher amount of capillary pores from attached mortar
  - ◎ Unhydrated cement particles
- ◎ Previous pavement design methods have not taken this into account
- ◎ Pavement mechanics are now being utilized to capture effects

# DRYING SHRINKAGE AT 50% RH

- Significantly higher for RCA concrete
- Capillary porosity exists in RCA and new mortar
- Crushing



# RESTRAINED SHRINKAGE STRAINS





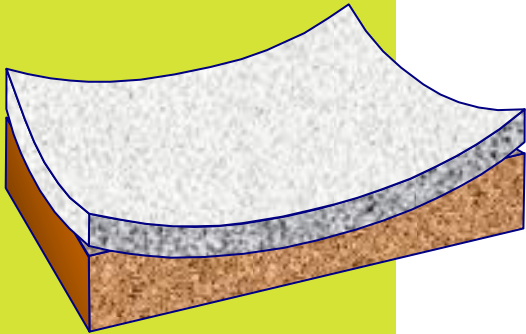
# RCA CONCRETE SLABS UNDER RESTRAINT

- ◎ While drying shrinkage is higher in RCA concretes ...
  - ◎ Increased creep characteristics of RCA concrete can relax strains at early ages
  - ◎ At later ages, creep is less effective in controlling strain magnitudes and deflections
- ◎ Rigid pavement is restrained from dowels, tie bars, slab-base friction/bonding, and self-weight

# MOISTURE IN CONCRETE PAVEMENT SLABS

- ◎ Due to this semi-porous nature of concrete
  - ◎ Moisture can get in and out of pores
  - ◎ Due to simplicity of pavement geometry, typically through surface
- ◎ Below depth of 50-100mm in concrete slab
  - ◎ Moisture content is nearly saturated and consistent
- ◎ Top 50-100mm varies with rain events, ambient relative humidity, wind, etc.
  - ◎ Leads to highly non-linear moisture gradients in slab

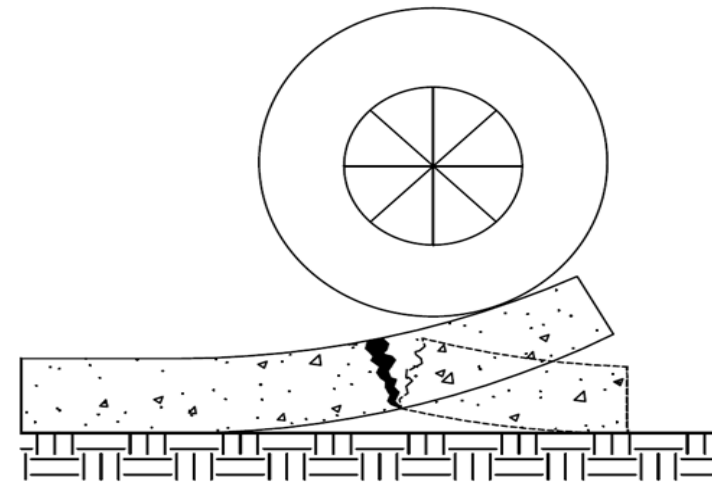
# MOISTURE LOSS EFFECTS



- ◎ Occurs through both self-desiccation and drying
  - ◎ Both cause volumetric changes
  - ◎ Autogenous (self-desiccation) happens throughout concrete
  - ◎ Drying shrinkage is a gradient within concrete
- ◎ Drying shrinkage and warping are linked
  - ◎ Tied to gel pores and smaller capillary pores
  - ◎ Differential drying shrinkage – permanent
  - ◎ Warping – reversible portion of shrinkage

# WHY CURL AND WARP MATTER

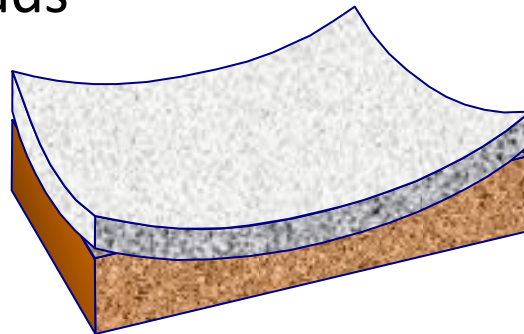
- ⊙ Stresses induced by environmental loading can be enough to crack a slab
- ⊙ Slab corners can be unsupported
- ⊙ Changing boundary conditions
- ⊙ Change in primary failure mechanism
- ⊙ Premature fatigue failure



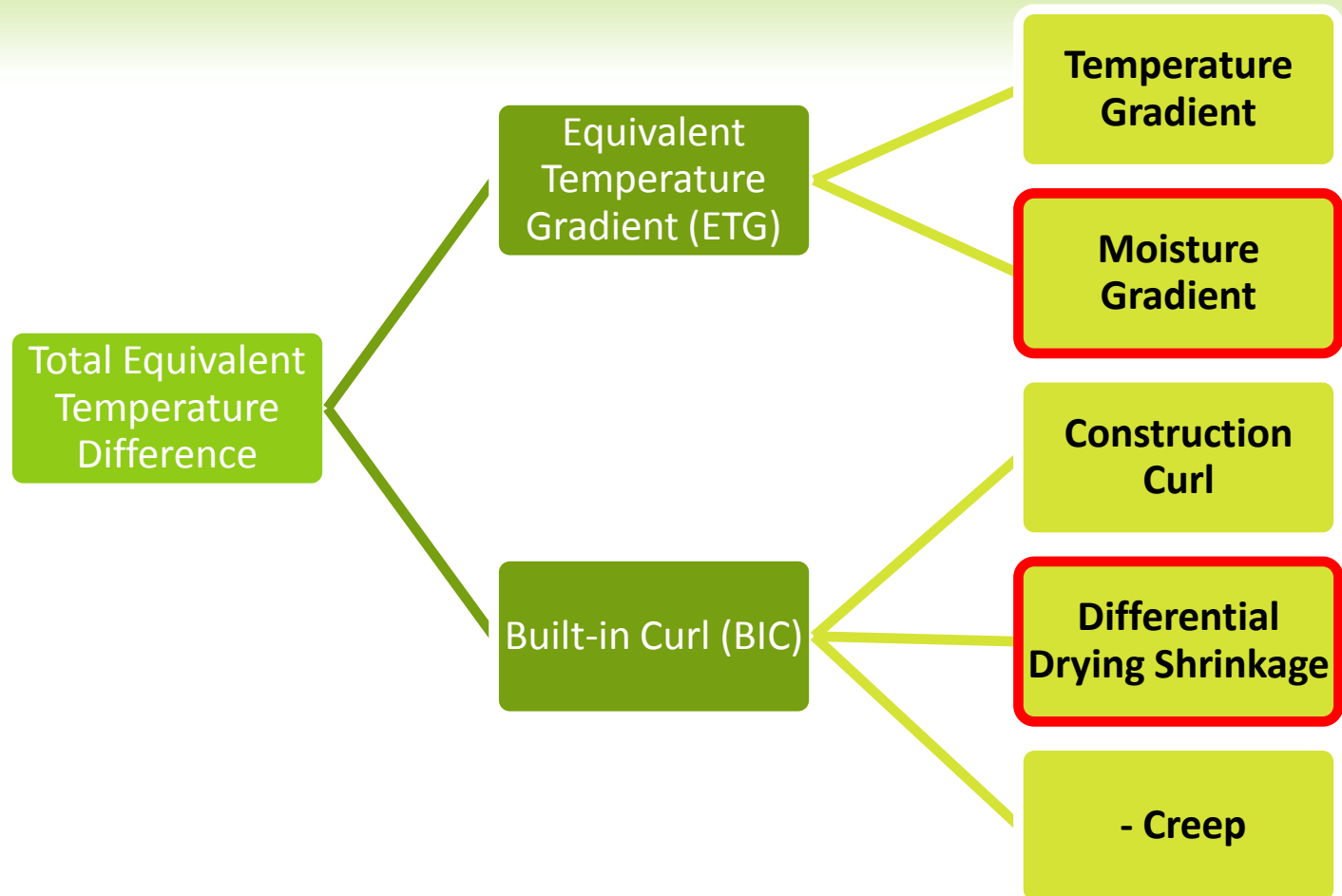
Top-down cracking from combined environmental and traffic loading

# DRYING SHRINKAGE LEADING TO SLAB DEFORMATION

- ◎ Warping and built-in curling issues can lead to alternative fatigue cracking development
- ◎ Gaps under slab corners
  - ◎ Interaction with external loads



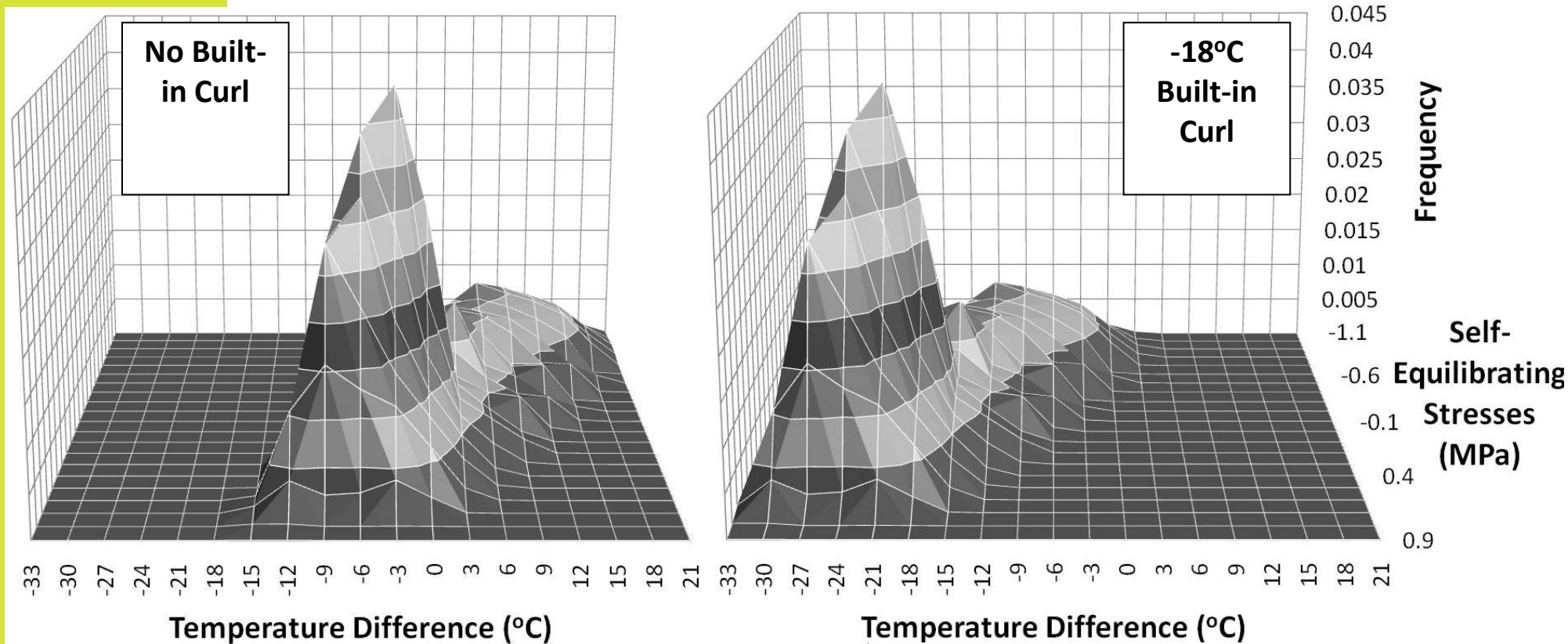
# VOLUME CHANGE COMPONENTS IN THE MECHANISTIC-EMPIRICAL PAVEMENT DESIGN GUIDE (MEPDG)





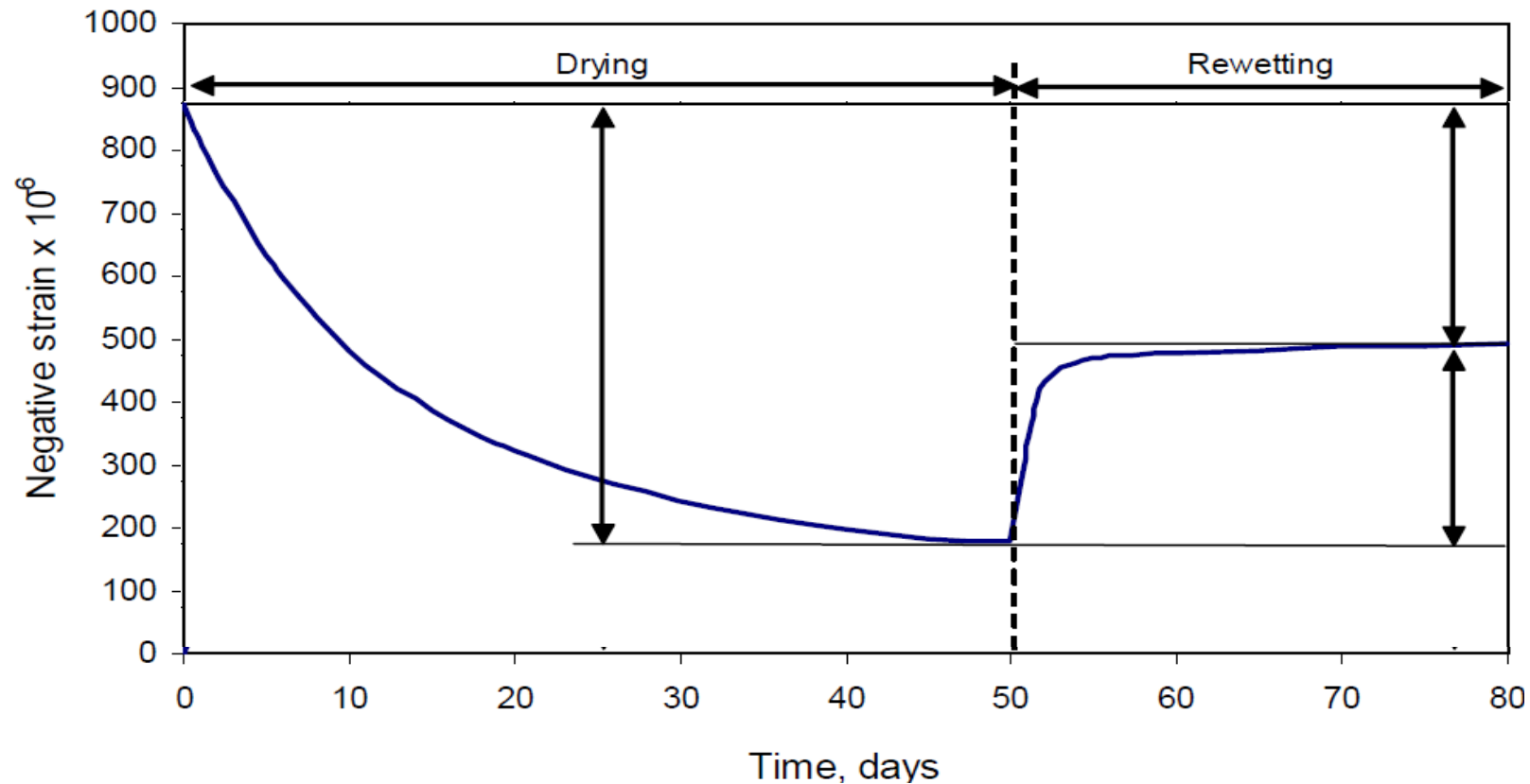
# TEMPERATURE SHIFT DUE TO BUILT-IN CURL

- ③ Shift can be large enough to never “curl down”



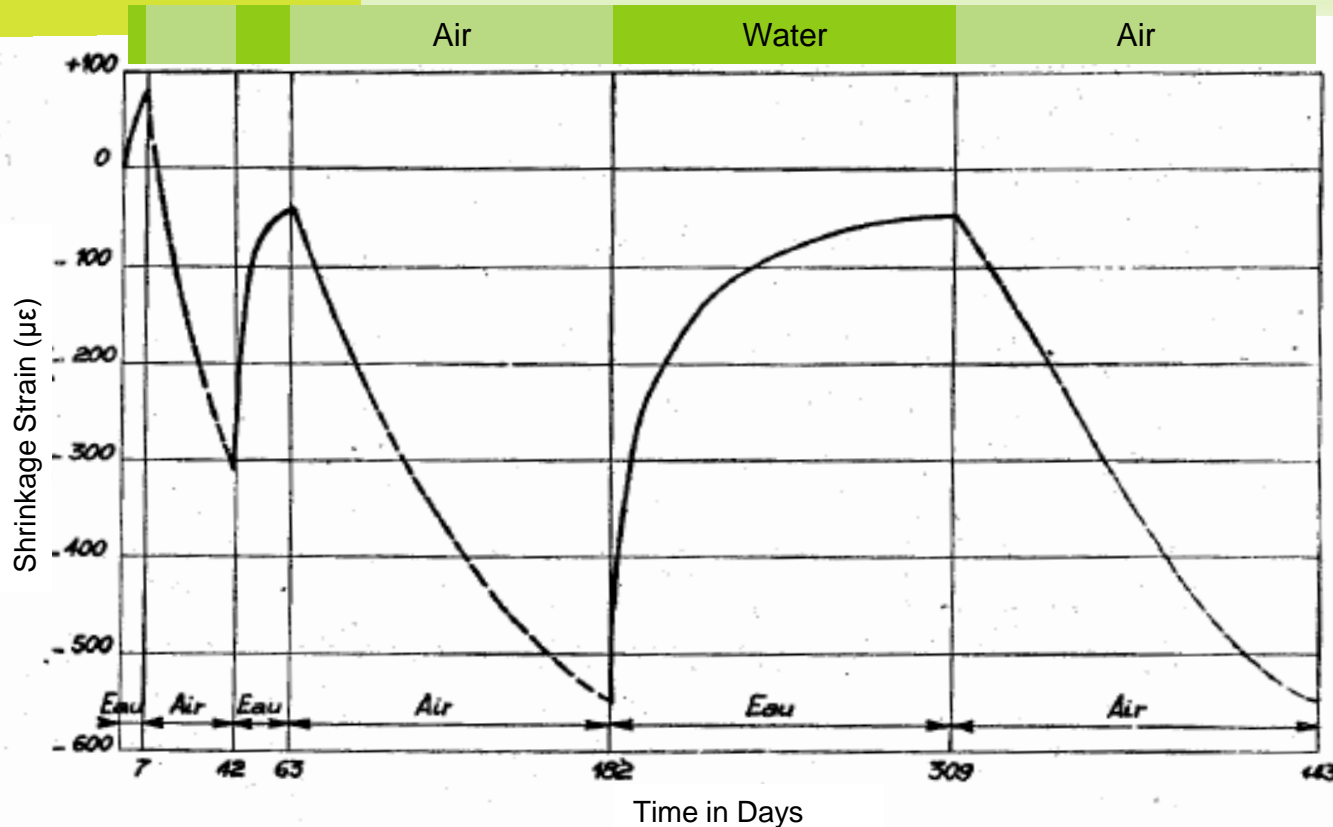
# REVERSIBLE SHRINKAGE - INTRODUCTION

- ◎ Portion of shrinkage is reversible
- ◎ Assumed to be 50% in MEPDG



Mindness and Young,  
(1981.) *Concrete*.  
Prentice Hall, Englewood  
Cliffs, NJ.

# REVERSIBLE SHRINKAGE - PAST RESEARCH



L'Hermite, R.L. (1947) "Le Retrait Des Ciments, Mortiers et Betons." Laboratoires du Batiment et des Travaux Publics.

- ◎ Some research in Europe from 1940-1975
- ◎ 40-70% of shrinkage is reversible
- ◎ Mortars stored in water for an extended cure had complete shrinkage reversibility

# MECHANISM OF REVERSIBLE SHRINKAGE

- ◎ The mechanism is not well understood
- ◎ Neville hypothesized that C-S-H gels form bonds when they are in close proximity during drying phase
- ◎ When the concrete is again exposed to moisture, these bonds swell, but hold the matrix together
  - ◎ Preventing shrinkage from being fully reversible

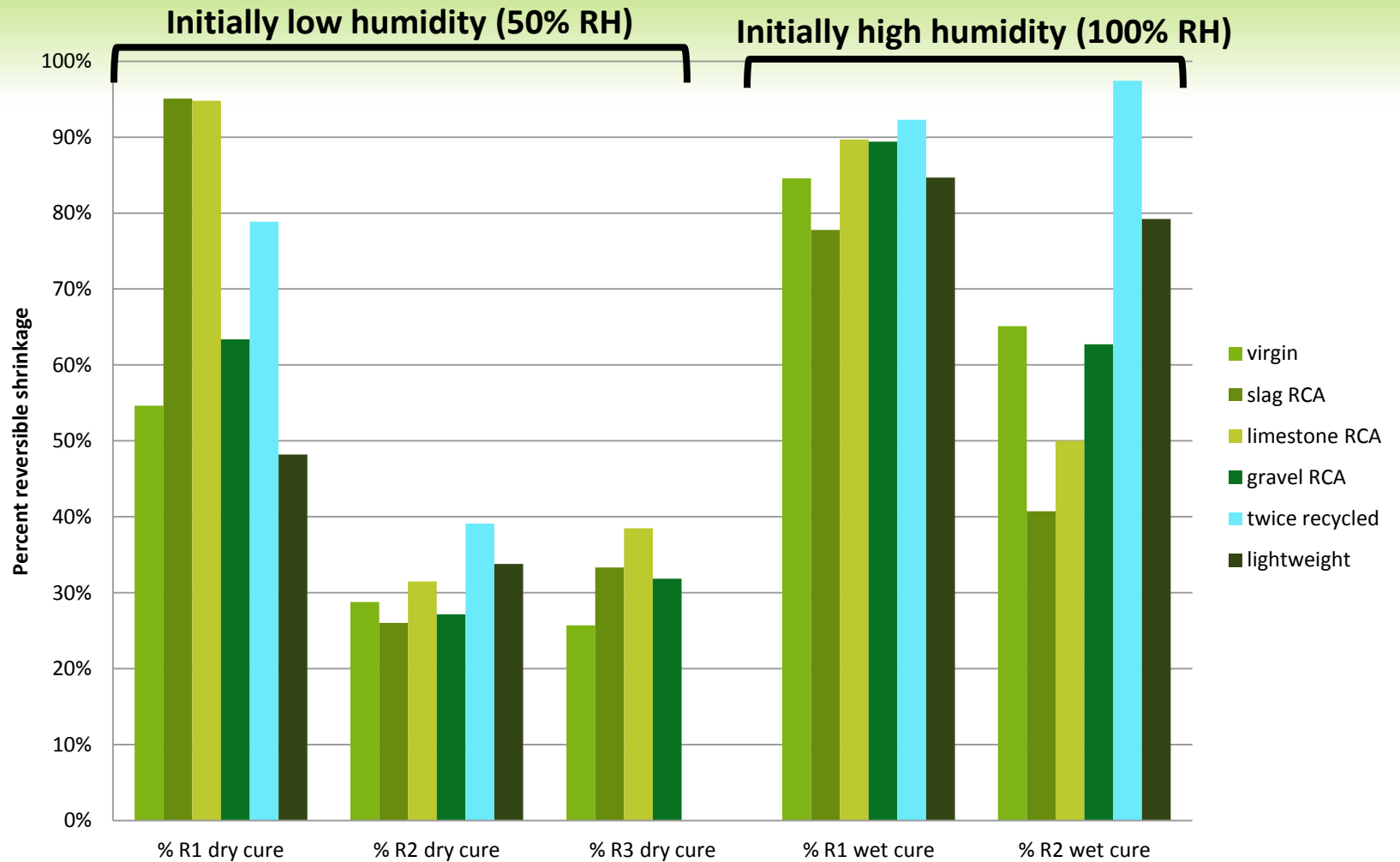
# MECHANISM OF REVERSIBLE SHRINKAGE

Granger L., Torrenti J.M., and Acker P., Thoughts About Drying Shrinkage: Scale Effect and Modelling. *Materials and Structures*, 1997. 30(3): pp. 96-105.

Sellier A., and Buffo-Lacarrière L., "Vers une Modélisation Simple et Unifiée du Fluage Propre, du Retrait et du Fluage en Dessiccation du Béton. *Revue Européenne de Génie Civil*, 2009. 13(10).

- ◎ Others hypothesize that a portion of irreversible shrinkage is related to microcracking
- ◎ During the shrinkage phase, microcracks are formed and open
- ◎ During swelling, cracks close either partially or fully
- ◎ Because these cracks cannot be reversed, some portion of drying shrinkage is believed to be irreversible

# % REVERSIBLE BY AGG TYPE





# WARPING & DDS - FINAL MODELS

Lederle, R.E., and Hiller, J.E., Development of New Warping and Differential Drying Shrinkage Models for Jointed Plain Concrete Pavements Derived with a Nonlinear Shrinkage Distribution, Accepted to the Transportation Research Record, 2012.

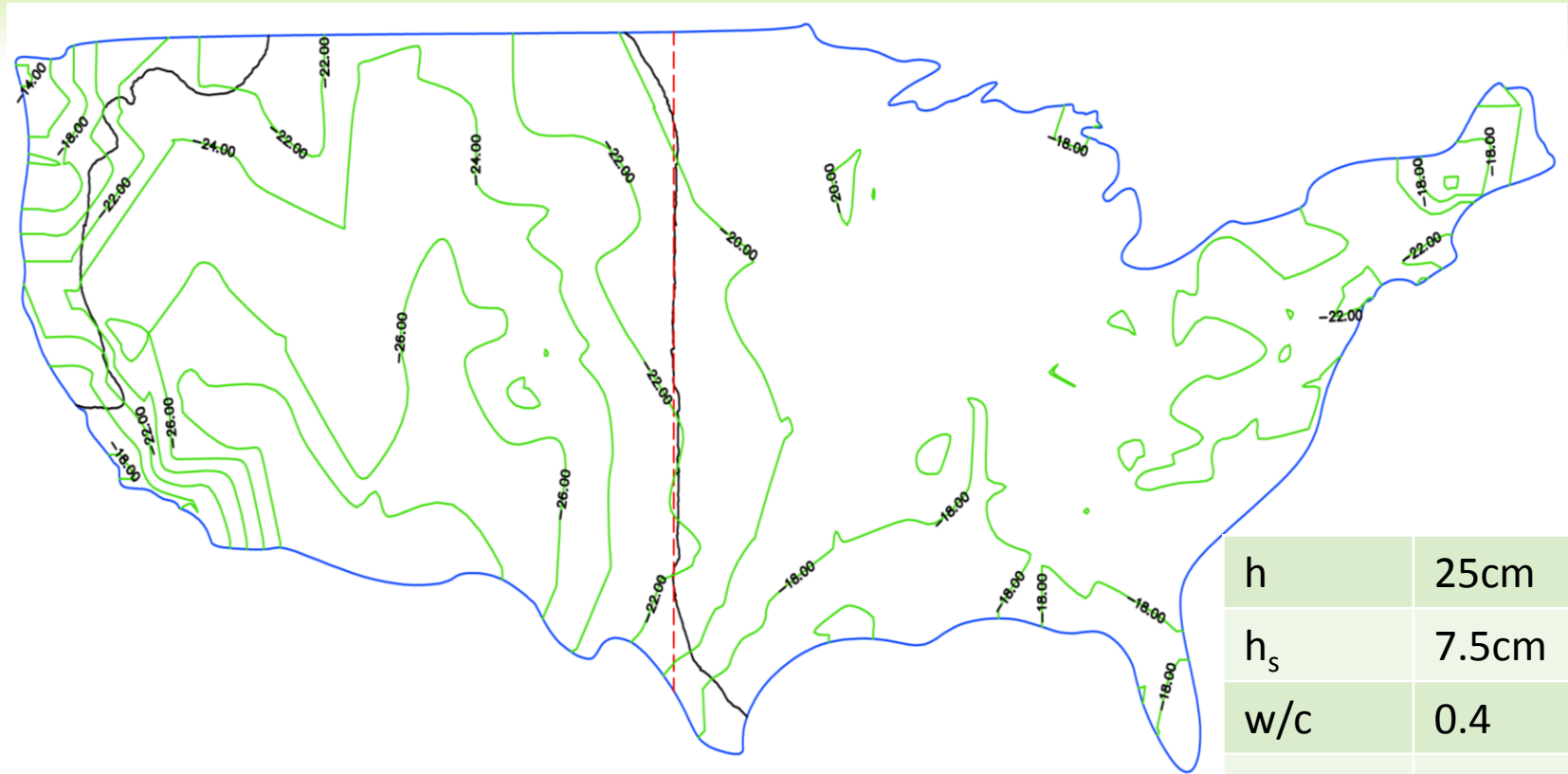
- ⊙ Non-linear model for equivalent temp. difference due to drying shrinkage in concrete pavements
- ⊙ Warping model

$$ETG_{warp} = \frac{\phi R \omega \varepsilon_u h_s [-3h(-4 + \pi) - 20h_s + 6\pi h_s](1 - \mu)}{2h^2 \alpha}$$

- ⊙ Differential Drying Shrinkage model

$$ETG_{DDS} = \frac{(1 - \phi) R \omega \varepsilon_u h_s [-3h(-4 + \pi) - 20h_s + 6\pi h_s](1 - \mu)}{2h^2 \alpha}$$

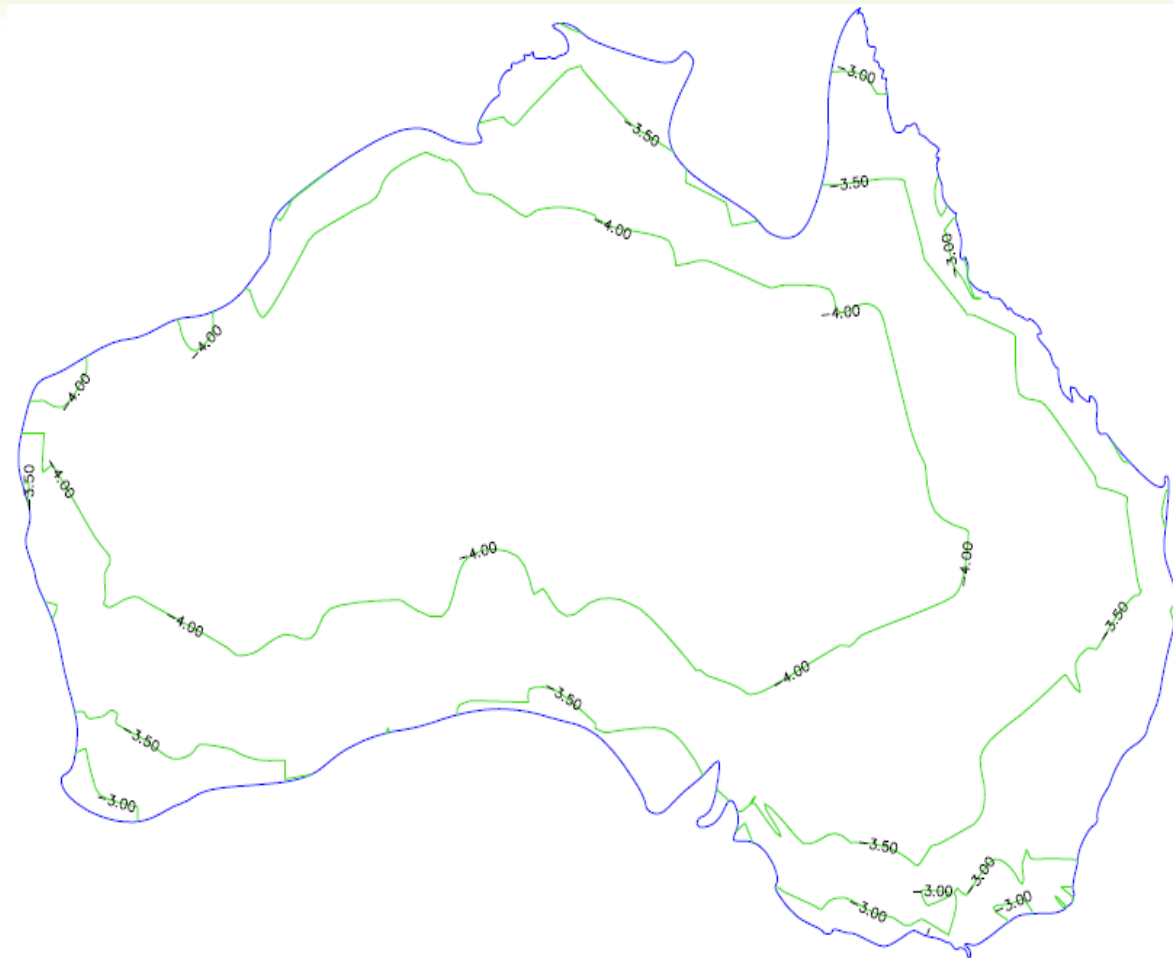
# WARPING & DDS - DESIGN AIDS



$h$	25cm
$h_s$	7.5cm
w/c	0.4
$\phi$	0.5
$\epsilon_{su}$	1200 $\mu\epsilon$

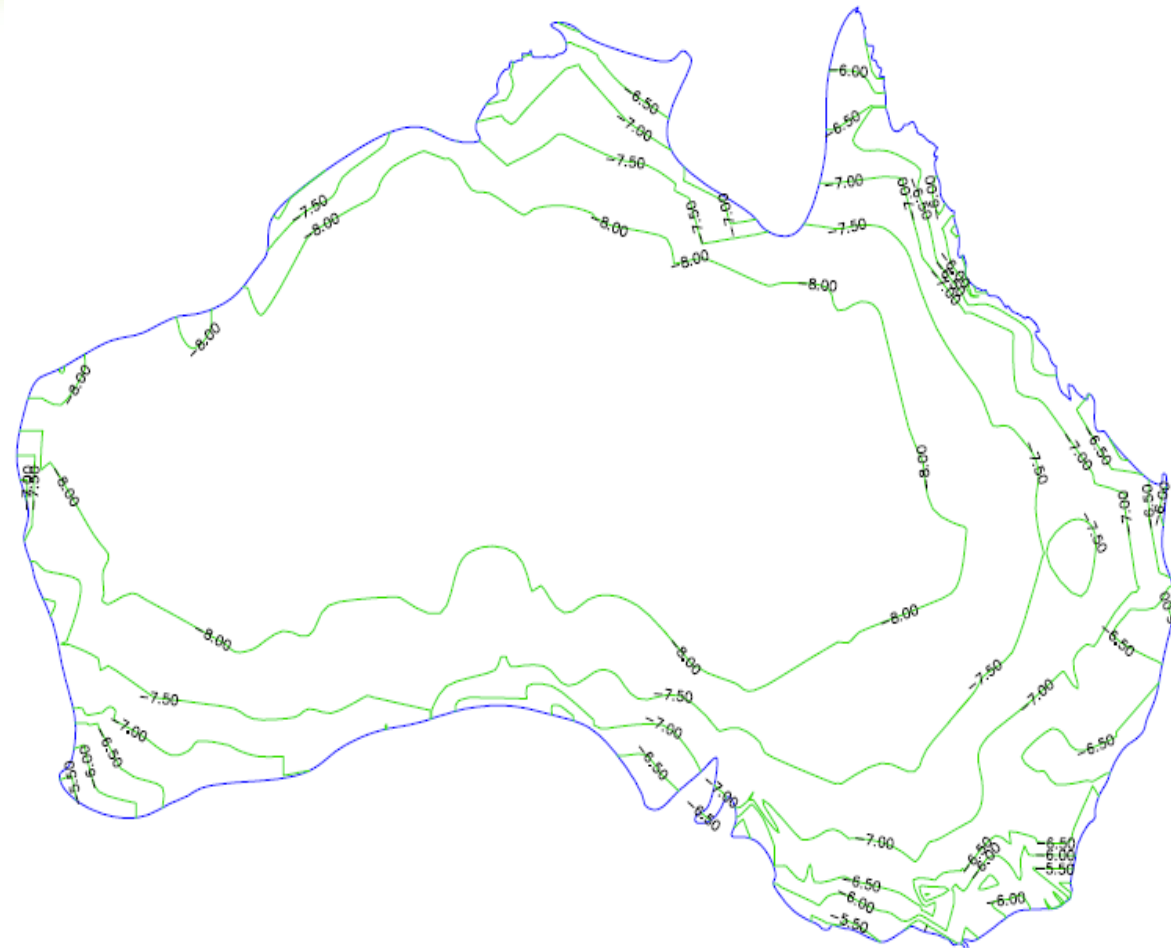
# HOW THIS AFFECTS CONCRETE PAVEMENTS IN AUSTRALIA

- © Non-linear model for equivalent temp. difference due to drying shrinkage



# HOW THIS AFFECTS CONCRETE PAVEMENTS IN AUSTRALIA

- ① For RCA concretes
- ① Higher drying shrinkage
- ① More change in coastal regions



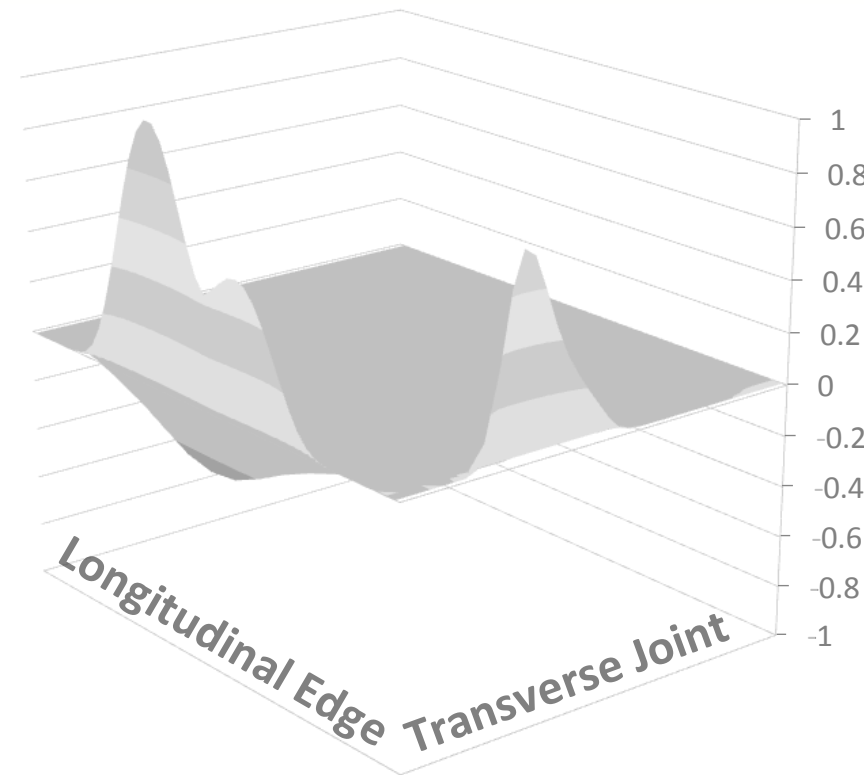
# SIGNIFICANCE OF HIGH LEVELS OF PERMANENT DIFFERENTIAL DRYING SHRINKAGE?

- ◎ Poor curing → High irreversible drying shrinkage
  - ◎ Leads to high levels of built-in curling in JPCP
  - ◎ Essentially causes equivalent temperature shift for specific climatic location
- ◎ MEPDG captures this effect to some degree
  - ◎ Only predicts transverse fatigue cracking
- ◎ To account for fatigue as transverse, longitudinal, or corner cracking →



# RADICAL

- ⊙ Mechanistic rigid pavement analysis tool
- ⊙ Predict fatigue cracking at multiple locations
  - ⊙ Transverse, longitudinal, corner cracking
  - ⊙ Top-down or bottom-up
- ⊙ Site-specific conditions, traffic, materials



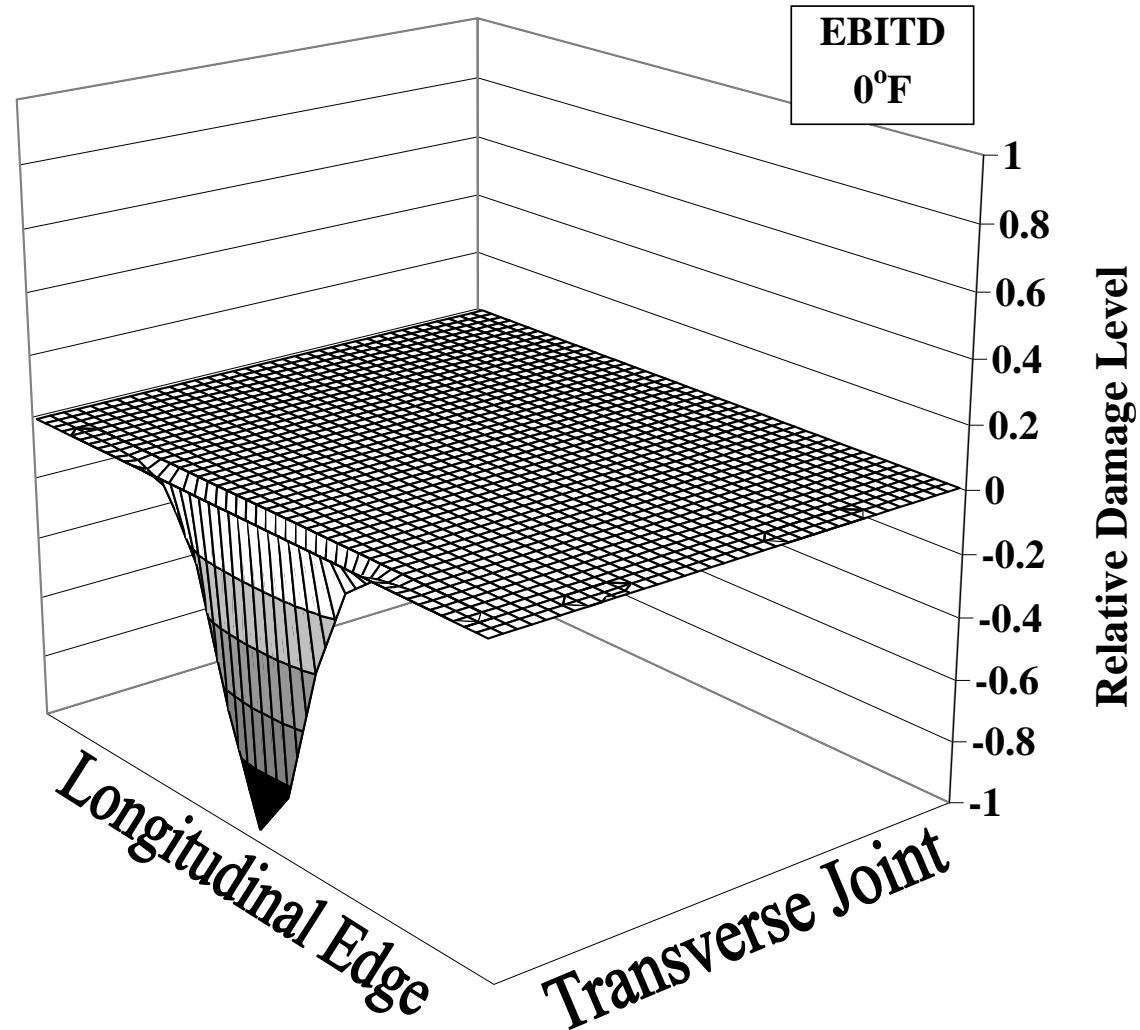


# LOW LOAD TRANSFER AND 4.6M JOINT SPACING

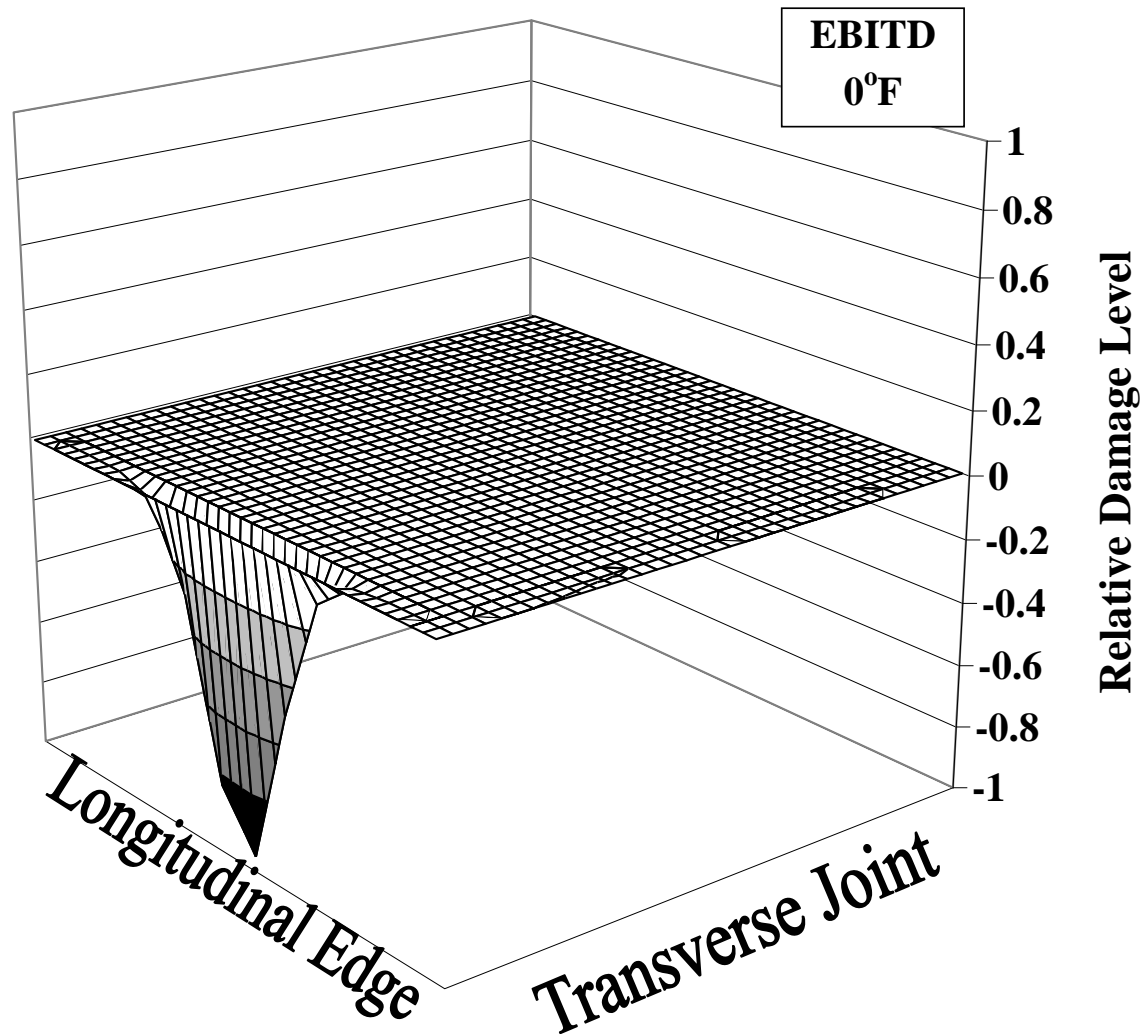
EBITD = Equivalent Built-in Temperature Difference

For temperature differences,  $^{\circ}\text{C} = 5/9 \text{ }^{\circ}\text{F}$

For San Francisco climate, traffic, soils, etc.



# LOW LOAD TRANSFER AND 3.7M JOINT SPACING



# RCA CONCLUSIONS

- ◎ More unpredictable fresh and hardened properties using RCA in concrete
  - ◎ Need to account for existing air content in RCA
- ◎ RCA concrete shows higher drying shrinkage, but better creep characteristics
  - ◎ Account for some factors in mechanistic design methods
  - ◎ Creep may be advantageous in restrained pavements
- ◎ Must understand differences and plan for multi-generational use → **Promote Sustainable Practices**

# RCA IN PVTs CONCLUSIONS

- ◎ Past pavement design/analysis has not taken moisture warping into consideration directly
  - ◎ Not site-specific design
  - ◎ MEPDG attempts to account for this ... unsuccessfully
- ◎ Amount of reversible warping can be controlled
  - ◎ Macro-level → curing conditions, w/c ratio, SCMs
  - ◎ Affects nano/micro-level → C-S-H gel spacing, micro-cracking
  - ◎ RCA concretes show no difference with virgin agg concretes
- ◎ Now have ability to capture these effects in pavement design, analysis, and construction practices

# RCA OPPORTUNITIES IN RIGID PAVEMENTS

- ◎ Cost savings for right projects
  - ◎ Typically urban with crushing on-site
  - ◎ Michigan environmental policy to use RCA if up to 10% greater cost than virgin aggregates
- ◎ Knowledge and control of original project aggs
- ◎ Lower cement content can yield good performance
- ◎ Design adjustments to accommodate differences
  - ◎ Joint spacing, thickness, limited restraint, materials?
  - ◎ Two-lift construction?



THANK YOU ...  
ANY QUESTIONS?

***MichiganTech***

